

Draft SR 99 Bored Tunnel Alternative Design Deviation No. 1: SR 99 Shoulder Width (Inside & Outside)

MP 30.40 to MP 32.83

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April 2009

The Alaskan Way Viaduct & Seawall Replacement Program

Draft SR 99 Bored Tunnel Alternative Design Deviation No. 1:

SR 99 Shoulder Width (Inside & Outside)

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Agreement No. Y-9715

Task CE.04

The Alaskan Way Viaduct & Seawall Replacement Program is a joint effort between the Federal Highway Administration (FHWA), the Washington State Department of Transportation (WSDOT), and the City of Seattle. To conduct this project, WSDOT contracted with:

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SR 99: Alaskan Way Viaduct & Seawall Replacement Program Draft SR 99 Bored Tunnel Alternative

Design Deviation No. 1: SR 99 Shoulder Width (Inside & Outside) MP 30.40 to MP 32.83

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Northwest Division Urban Corridors Office Seattle, Washington



Deviation Recommended for Approval:

Date_____

By_____, P.E.

Matt Preedy, Deputy Project Director

Deviation Approval:



Washington State Department of Transportation

Date _____

, P.E. Ed Barry, Assistant State Design Engineer for By ____ UCO

Deviation Approval:



U. S. Department of Transportation Federal Highway Administration

Date _____

By_

Randy Everett, Federal Highway Administration

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6 1.0 Project Overview

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7	The Alaskan Way Viaduct and Seawall Replacement Program (AWVSRP) design
8	team has been working since 2001 to develop alternatives for the replacement of the
9	Alaskan Way Viaduct. The team prepared and published a Draft Environmental
10	Impact Statement (Draft EIS) in March 2004 and a Supplemental Draft
11	Environmental Impact Statement (Supplemental Draft EIS) in September 2006. The
12	team also prepared and submitted a Design Approval Package (DAP) to the
13	Washington State Department of Transportation (WSDOT) for the preferred tunnel
14	alternative in February 2007; however, the approval of that package was deferred
15	while the project partners (City of Seattle, WSDOT, King County, and the U.S.
16	Federal Highway Administration (FHWA)) re-evaluated the preferred configuration
17	of State Route (SR) 99 in Seattle's waterfront area.
18	On March 14, 2007, the Project Team was directed by WSDOT to advance portions
19	of the program that would contribute to improving safety and mobility, and that
20	have fundamental consensus among the project partners. The Governor and
21	WSDOT then identified six "Moving Forward: Early Safety and Mobility Projects"
22	that are currently being implemented while the preferred \$R 99 configuration in the
23	central waterfront area was re-evaluated. One of these projects is the South Holgate
24	Street to South King Street Viaduct Replacement, which is currently under design
25	and scheduled for construction in fall 2009.
26	In January 2009 a letter of agreement between WSDOT, King County, and the City
27	of Seattle identified a four-lane, single-bore tunnel as the recommended alternative
28	for replacing the Alaskan Way Viaduct. The Single-bore Tunnel Alternative (Tunnel
29	Alternative) connects to the proposed South Holgate Street to South King Street
30	Viaduct Replacement Project (South Project) at approximately S. Royal Brougham
31	Way (M.P. 30.40) in the south with a cut-and-cover section that runs to First Avenue
32	and S. King Street. From here a tunnel boring machine would be used to construct
33	the tunnel, following First Avenue to approximately Stewart Street, then veering east
34	to the area of John Street and SR 99. The alternative includes another cut-and-cover
35	section that connects to SR 99 near Mercer Street (M.P. 32.83) in the north.
36	Interchange ramps are provided at both the north and south sections. Additionally,

1 2	a street connection between Alaskan Way and Elliott and Western Avenues is included (see Figure 1).
3	The SR 99 roadway for the SR 99 Bored Tunnel Project is functionally classified as a
4	Principal Arterial Highway by WSDOT; its geometric design classification is that of a
5	Principal Arterial P-1 Urban, per current WSDOT Design Manual Figure 440-6 (see
6	approved SR 99 Corridor Analysis Addendum C). The project corridor has a WSDOT
7	freight tonnage designation of T-1 (more than 10 million tons per year), and the City
8	of Seattle classifies the roadway as a Major Truck Street.
9	Design Matrix 3, lines 3-7 (WSDOT Design Manual Figure 325-5) (May be Matrix 3,
10	lines 3-11) is most applicable to the project, which requires a full design level.
11	Standard shoulder widths for a P-1 design class at a full design level are 10 feet on
12	the right and 4 feet on the left (WSDOT Design Manual Figure 440-6). This
13	document requests a deviation for reduced shoulder widths.
14	The AWVSRP is partially funded through a combination of state funds from the
15	2003 Nickel Funding Package and the 2005 Transportation Partnership Account
16	Package. It has also received funding from FHWA and the City of Seattle.



Analysis)



2 2.0 Existing Conditions in Vicinity of SR99

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The existing SR 99 urban route within the project vicinity is located along the waterfront between S. King Street and Pine Street before turning northeast to the Battery Street Tunnel (BST) at First Avenue and Battery Street. It exits the BST at Denny Way and then turns north, crossing John, Thomas, Harrison, Republican, and Mercer Streets. The majority of the existing SR 99 runs along the waterfront and therefore parallels Alaska Way directly to the west. To the east, the viaduct closely shadows downtown buildings and Western Avenue with general parking directly underneath.

11 The existing SR 99 through the project vicinity is generally configured as a 40-foot-12 wide viaduct stacked structure. The number of existing lanes ranges from three to 13 four in each direction, with lane widths varying from 9 to 12 feet and shoulder 14 widths ranging from 0 to 3 feet. The SR 99 roadway width narrows to 25 feet with 15 two lanes in each direction through the BST, and then widens to three to four lanes 16 each approximately 10 feet wide north of the BST. WSDOT currently considers the 17 BST a high accident location. The vertical alignment of the viaduct varies from 0 18 percent to 4 percent slope up to the BST, then SR 99 adjusts to approximately -2 percent north of the BST to Mercer Street before transitioning to a positive slope 19 20 northward. The rolling terrain between Union and Ward Streets has a posted speed 21 of 40 miles per hour (mph), while the southern portion of the route from S. Royal 22 Brougham Way to Union Street is level and posted as 50 mpb.

23 Many aspects of the existing horizontal and vertical curves do not meet today's 24 roadway design standards for the posted speed limit. When compared to current 25 design standards for stopping sight distance, horizontal curve radius, and vertical 26 curve length, about two-thirds of the horizontal and vertical curves would coincide 27 with a design speed of less than 40 mph.

The average daily traffic (ADT) peaks along the existing central waterfront mainline 28 29 at 52,500 for the northbound direction and 50,500 for the southbound direction. In 30 this area, the levek of-service (LOS) for northbound and southbound mainline traffic 31 varies between LOS D and LOS E. Existing ADT truck traffic volumes are approximately 3 percent to 5 percent of total traffic. At the northern end of the 32 33 project corridor, two existing High Accident Locations have been identified at northbound MP 31.9 to 32.1 and southbound MP 32.0 to 32.4 (see SR 99 Corridor 34 35 Analysis, October 2004).



2	3.0 Proj	posed Roadway Configuration
3		The SR 99 Bored Tunnel Alternative would replace the existing viaduct and BST
4		with a single bored tunnel east of the existing alignment. This project is comprised
5		of a bored tunnel containing two stacked roadway decks with cut-and-cover sections
6		at both the north and south ends. The tunnel would be constructed with an
7		approximately 54-foot diameter tunnel boring machine (TBM). Southbound traffic
8		would be on the top deck, and northbound traffic would be on the bottom deck.
9		Enclosed roadways would meet or exceed current fire, life, and safety codes. The
10		inside of the tunnel would be lined with an approximately 2-foot-thick concrete liner.
11		The alignment would consist of a minimum of two northbound and two southbound
12		lanes with shoulders varying on the left from 2 to 4 feet, and on the right from 3 to
13		8 feet. Both the South and North Segments of the project would contain fully
14		directional interchange movements connecting with the City's surface street grid.
15	3.1 Sou	th Segment
16		In the south, the alignment matches the S. Holgate Street to S. King Street Project
17		structure at S. Royal Brougham Way (RBW). SR 99 includes two fanes northbound
18		and two lanes southbound, with shoulder widths of 4 feet on the left and 8 feet on
19		the right (see Figure 2).
20		Access to and from the north would be via an interchange at RBW and the Alaskan
21		Way frontage road. The northbound on-ramp would enter a retained cut section
22		north of RBW and merge with the two SR 99 northbound lanes from the right side.
23		The southbound off-ramp would diverge from the left side of SR 99 (see Deviation
24		No. 2) and enter a retained cut section as it approaches RBW from the north.
25		Access to and from the south would be via an interchange at Alaskan Way north of
26		RBW. The southbound on-ramp would enter the two SR 99 southbound lanes from
27		a retained fill section as an added third southbound lane on the right. The
28		northbound off-ramp would exit SR 99 as right-side drop lane with two lanes
29		remaining on the SR 99 mainline, and would approach Alaskan Way on a retained fill
30		section.
31		Ramp design speeds, grades, and cross-sections are within WSDOT Design Manual
32		guidelines.







3.2 **Central Segment**

5	The bored tunnel's diameter of 54 feet was determined by setting two stacked
6	roadway widths at 36 feet, with 16.5 feet of vertical clearance each, and building out
7	structurally from there. Any additional clearance requirements would likely have a
8	direct impact on the tunnel bore's outside diameter. The roadway cross-section is
9	designed with a wall-to-wall width of 36 feet in both the northbound and
10	southbound directions. Two 12-foot lanes take up 24 feet, leaving 12 feet for
11	shoulders and possible barriers. There is ongoing discussion with regards to the
12	need of barrier shape within the tunnel (see Appendix A). Currently the project is
13	assuming the use of a barrier shape with a width of 9 inches for each barrier, the
14	remaining area allows for approximately 3 feet-3 inches for left shoulders and 7 feet-
15	3 inches for right shoulders (see Figure 3). Southbound traffic would be on the top
16	roadway deck, and northbound traffic would be on the lower roadway deck.

As part of the fire and life safety requirements, emergency egress locations would be located at approximately 600-foot intervals. To provide sufficient space for safe egress, the current design would require the northbound shoulder to be reduced to

- 13 feet minimum. The egress structures would be approximately 100 feet long (see2Figure 4).
- The horizontal and vertical design speed is 50 mph, per *WSDOT Design Manual*guidelines.



Alaskan Way Viaduct and Seawall Replacement Program SINGLE BORED TUNNEL CONCEPT November 4, 2008 Upper Walkway Upper Walkway Upper Roadway **Upper Roadway** SB 16.5 Continuous Upper Walkway 4 hour Fire Door Railing along black 54' NB Continuous <u>Lowe</u>r Walkway 11' 4 11 Continuous Walkway Continuous M IIIIIIII Walkway Lower Roadway Lower Roadway



Figure 4. Proposed Tunnel Section with Emergency Egress

3.3 North Segment

The stacked roadways of the bored tunnel section would begin to unbraid and unstack north of John Street, entering a cut-and-cover section between John and Harrison Streets, and a retained cut section north of Harrison Street. Northbound and southbound SR 99 would meet the existing vertical grade between Republican and Mercer Streets. SR 99 would follow and match the existing alignment from Mercer to Ward Streets. The existing SR 99 north of Mercer Street consists of three southbound lanes and three northbound lanes, with a fourth northbound auxiliary lane ending north of Aloha Street. The existing lane width varies from 10.5 feet to 11feet, and there are no existing shoulders, only a gore stripe on the right adjacent to the curb and a 6-foot sidewalk. The existing posted speed is 40 mph.

13Access to and from north SR 99 is via ramps at Harrison Street. A northbound on-14ramp joins to the two SR 99 mainline lanes as an additional third lane from the left15side at Republican Street (see Deviation No. 2). A southbound off-ramp exits from16SR 99 as a left-side drop lane (see Deviation No. 2) near Republican Street, leaving17two southbound mainline lanes (See Figure 5).

18Access to and from the south is via ramps at Republican Street. A southbound on-19ramp merges in to the two SR 99 mainline lanes from the right side of SR 99 in a20retained cut section. A southbound off-ramp diverges from the two SR 99 mainline21lanes in to a deceleration lane approaching Republican Street in a cut section.

SR 99 mainline horizontal and vertical design speed is 50 mph per WSDOT Design Manual guidelines. Ramp design speeds range from 10-45mph.





1 **4.0 Deviation Description – Shoulder Width**

The proposed design matrix for the project indicates a "Full Design Level" for
shoulder widths. *WSDOT Design Manual* Figure 440-6 specifies a 10-foot right
shoulder width and a 4-foot left shoulder width.

5 This deviation proposes shoulder widths along SR 99 between NB Sta. 178+00 and 6 Sta. 319+90, and SB Sta. 178+00 and Sta. 319+90, as shown in Figures 2, 3, 4 and 5. 7 Table 1 lists the design standard and proposed left and right shoulder widths for the 8 project.

	Left Shoulder Width		Right Shoulder Width	
	(1	feet)	(feet)
Direction	Standard		Standard	
	(Design Manual	Proposed	(Design Manual	Proposed
	Figure 440-6,		Figure 440-6	
	May 2008)		May 2008/	\searrow
Northbound	4 feet	178+00 to 200+00 4 feet	10 feet	178+00 to 200+00 8 feet
NT		200+00 to $285+00$		200+00 to 285+00
Northbound	4 feet	3 feet-3inches	10 feet 🧹	Varies 3 feet to 7
				feet-3 inches
		300 ± 00 to 317 ± 00	\sim	300+00 to $319+90$
Northbound	4 feet	300+00 to $31/+00$	10 feet	inches to 0 feet
		<u> </u>	(/ $ >$	to match existing
		317+00 to 319+90		317+00 to 319+90
Northbound	4 feet	Varies <mark>3 feet-3</mark>		Varies 3 feet-3
Northbound		inches to 0 feet to	10 feet	inches to 0 feet
		match existing		to match existing
Southbound	C	178+00 to 200+00	10 foot	178+00 to 200+00
bound	4 ieet	4 feet		8 feet
Southbound		200+00 to 285+00	40.5	200+00 to 285+00
Soumbound	4 feet	3 feet-3inches	10 feet	7 feet-3 inches
		\sim /		300+00 to 317+00
Southbound	4 feet	300+00 to 317+00	10 feet	Varies 7 feet-3
		<mark>3 feet-3 inches</mark>		inches to 3feet-3
		\sim		inches
		317+00 to 319+90	10 feet	317+00 to 319+90
Southbound	4 feet	Varies <mark>3 feet-3</mark>		Varies 3 teet-3
		Inches to 0 feet to		inches to 0 feet
		match existing		to match existing

Table 1: Shoulder Widths

1 Any additional horizontal and/or vertical clearance requirements would likely have a 2 direct impact on the tunnel bore's outside diameter, and require a reassessment of 3 current tunneling technology. Shoulder deviations are required at the north end of 4 the project to match existing SR 99 substandard conditions.



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5.0 Alternatives Considered

3	The Project Team developed and assessed alternate alignments that would best meet
4	WSDOT's design standards, minimize effects to properties, minimize impacts to the
5	environment, minimize construction impacts, work within the limitations of current
6	tunnel construction technology, and allow for efficient construction staging. The
7	alignment alternatives for the mainline roadway are summarized in the following
8	sections.

9 5.1 Alternative 1: Nonstandard Shoulder Width

10	Alternative 1 illustrates the preferred alignment. It maximizes shoulder widths,
11	provides 12-foot lane widths, stays within the proposed tunnel diameter, and
12	minimizes right-of-way impacts. This roadway configuration is illustrated in Figures
13	2, 3, 4 and 5. The tunnel design includes shoulder deviations for shoulder width as

- described in Section 4.0.
- 15(Talk about TBM expanding technology and will be largest built in US. Initially used16twin bore but due to costs, went with single bore going to upper limits of current17technology).

18Justificationfor reduced shoulder widths includes tunnel boring machine technology,19emergency egress location requirements by fire/life/safety codes, and matching20existing non-standard condition north of the project.

- 21The following constraints have been determined through preliminary design and22discussion with the Project Team:
- 23 Tunnel Boring Diameter Technology
 - Match existing substandard shoulder widths in the North Segment
 - Egress locations at 600 foot intervals in tunnel

26 Deviations from full design standard shoulder widths would avoid major 27 reconfiguration of the tunnel's proposed diameter and reduce costs.

- 28 5.2 Alternative 2: Full Design Standards Alternative
- Alternative 2 would apply a project design that used full design standards. This Full
 Design Standards Alternative was eliminated from further consideration because it
 would exceed the TBM diameter limits for the technology and would indefinitely
 extend the project limits where existing shoulder widths are nonstandard.

1An approximately 58 foot diameter bore would be required to provide the standard2shoulders of 4 feet and 10 feet. A 60 foot bore would be required to accommodate3the emergency egress and standard shoulders. Further increasing diameter could4prohibitively increase cost of construction and risk. Twin large diameter bores could5also be used to accommodate the space restrictions, however this would be cost6prohibitivel.



2 **6.0 Justification**

3 4	Alternative 1 (Preferred Alternative) proposes nonstandard design elements for the SR 99 Bored Tunnel Project as follows:
5	• Northbound shoulder width: Varies 2 to 4 feet (left) and 7 to 10 feet (right)
6	• Southbound shoulder width: Varies 2 to 4 feet (left) and 3 to 10 feet (right)
7	The justifications for recommending nonstandard design elements are as follows:
8 9 10 11	 The 54-foot-diameter single-bore tunnel is at the technological limit for the industry. The stacked roadway configuration within this diameter has horizontal width limitations when combined with the necessary two lanes, vertical clearance, emergency access points, and potential barriers.
12 13 14 15	2. The proposed shoulder widths must match the existing shoulder widths at the end of the project limits, which requires that a portion of the shoulder widths within the project limits be tapered down to match the non-standard shoulder widths of the existing roadway.
16 17	3. The horizontal stopping sight distances that are provided meet standards for a 50-mph design speed, which matches the design speed for the facility.
18 19 20 21 22 23 24 25 26 27	4. Shoulder widths on SR 99 were reduced south of Mercer Street to accommodate the project's mainline construction including on and off ramps and a construction detour carrying mainline traffic west of the project (See Corridor Operation Plan Report). Providing tull shoulders along this segment would increase the project width and require acquisition of additional properties either west or east of the project. These adjacent properties have buildings abutting the property line and any impacts would require full property acquisitions at a significant cost increase to the project (see Right of Way Impact Report).
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2 7.0 Recommendation

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Alternative 1 proposes a configuration that provides sufficient roadway facilities to fully accommodate vehicular traffic. This alternative also considers the urban environment and other users of the facilities, and balances pedestrian safety with appropriate roadway geometry. The proposed deviation would not adversely affect the safety or functionality of the vehicular traveled way. Impacts to adjoining property uses would also be minimized.

- 9 The Project Team recommends that shoulder widths of the mainline roadways be 10 deviated from design guidelines for the SR 99 Bored Tunnel Project. The Project 11 Team also recommends approval of these deviations based on the above 12 justifications.